

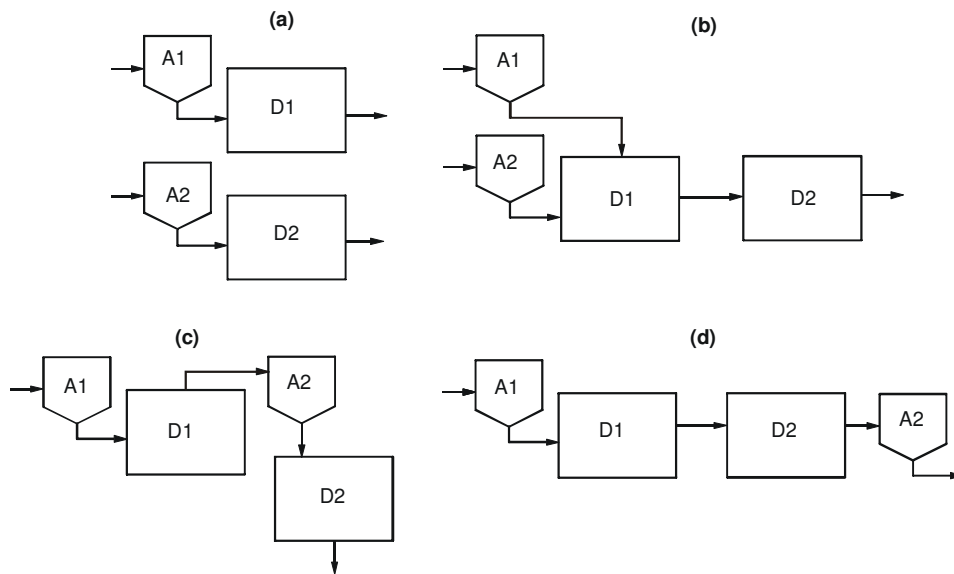
**Example 8.3**

An activated sludge system treats industrial effluents and produces  $40 \text{ ton TSS}\cdot\text{d}^{-1}$  of sludge ( $f_v = 0.75$ ) with an active fraction  $f_{av}$  of 0.6. The temperature is  $30^\circ\text{C}$  and the sludge exhibits good settling characteristics:  $k = 0.25 \text{ l}\cdot\text{g}^{-1}$  and  $v_0 = 12 \text{ m}\cdot\text{h}^{-1}$ . The system consists of three aerobic digesters of  $5000 \text{ m}^3$  each and two thickeners with a diameter of 18 m. Determine the optimal configuration to carry out sludge stabilisation. Calculate the required aeration power (assume that nitrification does not develop).

**Solution:**

In Fig. 8.11 several possible configurations for the sludge stabilisation are shown:

- Two parallel systems of thickener and digester;
- Two thickeners in parallel followed by two digesters in series;
- Two systems in series, each consisting of a thickener + digester;
- Thickener and two digesters plus a second thickener in series.



**Figure 8.11 Schematic representation of different configurations for thickening and digestion of the sludge in Example 8.3**

If all the sludge ( $40 \text{ ton}\cdot\text{d}^{-1}$ ) is discharged into a single thickener with a diameter of 18 m (the thickener area is  $314 \text{ m}^2$ ), the applied solids load is  $40,000/314 = 123 \text{ kg}\cdot\text{m}^{-2}\cdot\text{d}^{-1}$  or  $5.1 \text{ kg}\cdot\text{m}^{-2}\cdot\text{h}^{-1}$ . By equating this value to the limiting flux (i.e. with a  $s_f$  of 1), the concentration of the thickened sludge can be calculated:

$$F_l = F_{sol} = X_t \cdot (k \cdot X_l - 1) \cdot \exp(-k \cdot X_l) = 5.1 \text{ g TSS}\cdot\text{l}^{-1}$$

For a single thickener, the trial and error solution is  $28 \text{ g TSS}\cdot\text{l}^{-1}$ . In case of having two parallel thickeners, the applied solids load to each is  $2.55 \text{ g TSS}\cdot\text{l}^{-1}$  and the thickened sludge concentration increases to  $32 \text{ g TSS}\cdot\text{l}^{-1}$ .

As the applied sludge mass is  $40 \text{ ton}\cdot\text{d}^{-1}$ , the flow of thickened sludge is  $40,000/28 = 1428 \text{ m}^3\cdot\text{d}^{-1}$  for a single thickener and  $40,000/32 = 1250 \text{ m}^3\cdot\text{d}^{-1}$  for two thickeners in parallel. Since the digesters have a volume of  $5000 \text{ m}^3$ , the retention time per digester would be 3.5 and 4 days respectively.

In the case of configuration (a) of Fig. 8.11, the thickened sludge concentration is  $32 \text{ g TSS}\cdot\text{l}^{-1}$  and the flow of thickened sludge is divided over the two digesters so that each receives  $625 \text{ m}^3\cdot\text{d}^{-1}$ . Hence the retention time is  $R_d = 5000/625 = 8$  days and the active fraction in the digested sludge is given by Eq. (8.39):

$$\begin{aligned} 1/f_{ae} + f - 1 &= (1/f_{ai} + f - 1) \cdot (1 + b_h \cdot R_d) \\ &= (1/0.6 + 0.2 - 1) \cdot (1 + 0.36 \cdot 8) = 3.36 \text{ and } f_{ae} = 0.24 \end{aligned}$$

The digested active sludge concentration is:

$$\begin{aligned} X_{ad} &= X_{ai} \cdot (1/f_{ai} - 1/f_{ae}) / (1 - f - 1/f_{ae}) \\ &= 0.60 \cdot 0.75 \cdot 28 \cdot (1/0.6 - 1/0.24) / (1 - 0.2 - 1/0.24) = 9.4 \text{ g VSS}\cdot\text{l}^{-1} \end{aligned}$$

Hence the decrease of the volatile sludge concentration is:

$$X_{vd} = (1 - f) \cdot X_{ad} = 0.8 \cdot 9.4 = 7.5 \text{ g VSS}\cdot\text{l}^{-1}$$

It is concluded that the excess volatile sludge mass decreases by  $100 \cdot 7.5/28 = 27\%$ . In the case of configuration (b) of Fig. 8.11, the concentration of active sludge is again  $32 \text{ g TSS}\cdot\text{l}^{-1}$  and the retention time is  $R_{d1} = R_{d2} = 4$  days. Hence:

$$1/f_{ae} + f - 1 = (1/f_{ai} + f - 1) \cdot (1 + b_h \cdot R_{d1})^2 = (1/0.6 - 0.8) \cdot (1 + 0.36 \cdot 4)^2 = 5.2 \text{ and } f_{ae} = 0.17$$

The concentrations of digested active sludge and the decrease of volatile sludge in this configuration are  $X_{ad} = 12.2 \text{ g VSS}\cdot\text{l}^{-1}$  and  $X_{vd} = 9.8 \text{ g VSS}\cdot\text{l}^{-1}$  respectively. The decrease of volatile sludge in this configuration represents  $100 \cdot 9.8/32 = 30\%$  of the excess volatile sludge mass. In configuration (c) of Fig. 8.11, in the first thickener the flux is  $5.1 \text{ kg}\cdot\text{m}^2\cdot\text{d}^{-1}$  and the concentration of thickened sludge is  $28 \text{ g TSS}\cdot\text{l}^{-1}$  with  $R_{d1} = 3.5$  d. Thus the fraction of active sludge leaving the first digester is:

$$1/f_{ae1} + f - 1 = (1/f_{ai} + f - 1) \cdot (1 + b_h \cdot R_{d1}) = 1.96 \text{ and } f_{ae1} = 0.36$$

The concentration of digested active sludge is calculated from Eq. (8.50):

$$\begin{aligned} X_{ad1} &= X_{ai} \cdot (1/f_{ai} - 1/f_{ae1}) / (1 - f - 1/f_{ae1}) \\ &= 0.60 \cdot 0.75 \cdot 28 \cdot (1/0.6 - 1/0.36) / (1 - 0.2 - 1/0.36) = 7.0 \text{ g VSS}\cdot\text{l}^{-1} \end{aligned}$$

Thus the decrease of the sludge concentration in the first digester is (Eq. (8.51)):

$$X_{vd1} = (1 - f) \cdot X_{ad1} = 0.8 \cdot 7.0 = 5.6 \text{ g VSS}\cdot\text{l}^{-1}$$

The concentration of the sludge leaving the first digester is  $28 - 5.6 = 22.4 \text{ g TSS}\cdot\text{l}^{-1}$ , resulting in a flux of  $1428 \cdot 22.4/324 = 99 \text{ kg}\cdot\text{m}^2\cdot\text{d}^{-1}$  or  $4.1 \text{ kg}\cdot\text{m}^2\cdot\text{h}^{-1}$  in the second thickener. If it is assumed that the settleability does not change, the thickened sludge concentration in the second thickener can be calculated as  $29 \text{ g TSS}\cdot\text{l}^{-1}$ , so that the flow of sludge is then decreased from 1428 to  $1428 \cdot 22.4/29 = 1100 \text{ m}^3\cdot\text{d}^{-1}$ , and the retention time in the second digester becomes  $R_{d2} = 5000/1100 = 4.5$  days. Hence the final active fraction will be:

$$1/f_{ae2} + f - 1 = (1/f_{ae1} + f - 1) \cdot (1 + b_h \cdot R_{d2}) = 5.20 \text{ and } f_{ae2} = 0.17$$

The digested active sludge concentration and the decrease of volatile sludge are calculated as  $X_{ad} = 10.5$  and  $X_{vd} = 8.4 \text{ g.l}^{-1}$  respectively, i.e. a fraction of  $100 \cdot 8.4 / 28 = 30\%$  of the volatile sludge is mineralised.

In configuration (d) the situation in the first thickener is the same as in configuration (c). In the series of two digesters the final active sludge fraction is calculated as:

$$\begin{aligned} 1/f_{ae} + f - 1 &= (1/f_{ai} + f - 1) \cdot (1 + b_h \cdot R_{d1})^2 \\ &= (1/0.6 + 0.8) \cdot (1 + 0.36 \cdot 3.5)^2 = 4.42 \text{ and } f_{ae} = 0.19 \end{aligned}$$

The digested active sludge concentration is now calculated as:

$$\begin{aligned} X_{ad} &= X_{ai} \cdot (1/f_{ai} - 1/f_{ae}) / (1 - f - 1/f_{ae}) \\ &= 0.60 \cdot 0.75 \cdot 28 \cdot (1/0.6 - 1/0.19) / (1 - 0.2 - 1/0.19) = 10.0 \text{ g VSS.l}^{-1} \end{aligned}$$

Hence :

$$X_{vd} = 0.8 \cdot 10.0 = 8.0 \text{ g VSS.l}^{-1}$$

The solids flux to the second thickener will be  $1428 \cdot (28 - 8) / 324 = 88 \text{ kg.m}^{-2} \cdot \text{d}^{-1}$  or  $3.7 \text{ kg.m}^{-2} \cdot \text{h}^{-1}$ . For this value, the concentration of thickened sludge is calculated as  $29.8 \text{ g TSS.l}^{-1}$ : i.e. the flow of thickened digested sludge will be  $20 / 29.8 \cdot 1428 = 960 \text{ m}^3 \cdot \text{d}^{-1}$  (it is assumed that the digestion does not effect the settleability of the sludge).

**Table 8.4 Summary of the performance of the different thickening and stabilisation configurations of Example 8.3**

Parameter	UoM	Configuration (Fig. 8.11)			
		(a)	(b)	(c)	(d)
Residual active sludge fraction	-	0.24	0.19	0.17	0.19
Mass flow of stabilised sludge	ton TSS.d <sup>-1</sup>	30.6	28.6	28.0	28.6
Volumetric flow of stable sludge	m <sup>3</sup> .d <sup>-1</sup>	1428	1250	1100	960
Fraction mineralised volatile sludge	%	23	29	30	29

In Table 8.4 the results of the calculations are summarised. It can be concluded that option (a) is the most unattractive alternative: the stabilised sludge still has a fraction of 24 percent of active sludge, which is more than the 20 percent generally adopted as an acceptable upper limit, and the flow of stabilised sludge is also larger than in the other options:  $1428 \text{ m}^3 \cdot \text{d}^{-1}$ . The other three options are more or less equivalent in terms of the residual active fraction. However, the digested sludge flow (and hence disposal costs) of option (d) is the smallest and for that reason might be adopted in practice.